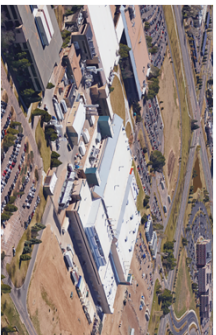


Wind Loads on Non-Building Structures for the Practicing Engineer

MN
SEA



Emily Guajalelino
May 25, 2021

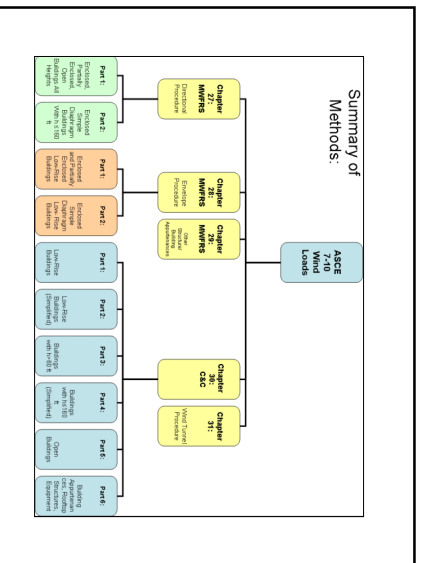
1

NCSEA Recommendations to ASCE 7:

1. Reduce Number of Methods to one (1) Computational Method and one (1) Tabular Method.
2. Consolidate Wind Provision from ASCE 7 and IBC into ASCE 7 and simplify the provisions.
3. Provide criteria for commonly encountered conditions (Canopies, Tall Parapets, Mechanical Screens, PV Panels).
4. Provide design procedures for RTUs on buildings > 60'.
5. Simplify free-standing wall provisions.
6. Provide guidance for irregular building configurations.



3



5

2011 NCSEA provided a survey to 9,500 engineers.
 “What modifications or additions would you like to see in the wind sections of ASCE 7?”

1
2
3
4
5

2

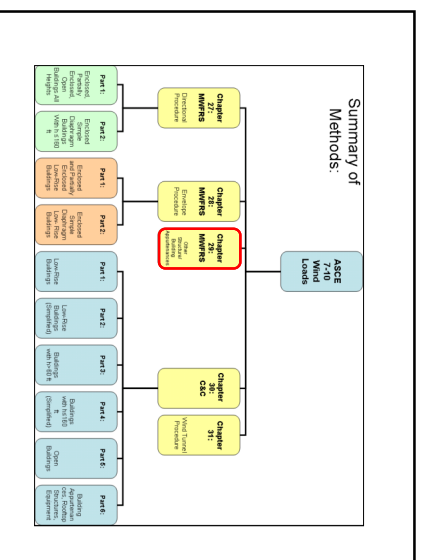
Topics:

WIND LOADS ON:

- Rooftop Equipment
- Solar PV
- Rooftop Screenwalls
- Freestanding Walls and Signs
- Tall Parapets
- Tanks and Silos
- Trellises
- Canopies



4



6

Main Wind Force Resisting System (MWFRS)

Chapter 29: Other Structures

- Conditions, Limitations
- Solid Freestanding Walls or Signs
- Solid Attached Signs
- Wind Loads on Rooftop Structures and Equipment
- Parapets
- Roof Overhangs
- Minimum Design Wind Loadings

ASCE 7-10

7

Main Wind Force Resisting System (MWFRS)

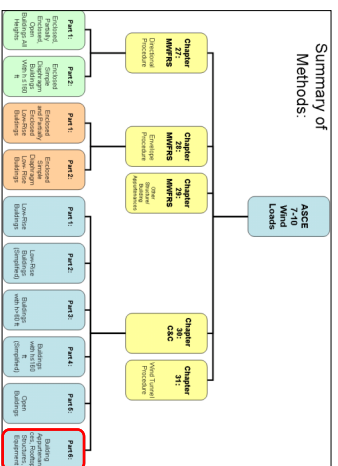
Chapter 29: Other Structures

- Scope, Conditions, Limitations
- Solid Freestanding Walls and Solid Signs
- Other Structures
 - Rooftop Structures and Equipment
 - Circular Bins, Silos, and Tanks
 - Rooftop Solar
 - Parapets
 - Roof Overhangs
 - Minimum Design Wind Loadings

ASCE 7-16

8

Summary of Methods:



9

Components & Cladding (C&C)

Chapter 30: Part 6:

C&C for Building Appurtenances and Rooftop Structures and Equipment

- Parapets
- Roof Overhangs
- Rooftop Structures and Equipment

ASCE 7-10

10

Components & Cladding (C&C)

Chapter 30: Part 6:

C&C for Building Appurtenances and Rooftop Structures and Equipment

- Parapets
- Roof Overhangs
- Rooftop Structures and Equipment
- Attached Canopies on Buildings < 60 ft

ASCE 7-16

11

Components & Cladding (C&C)

Chapter 30: Part 7:

C&C for Non-Building Structures

- Circular Bins, Silos, and Tanks with $h \leq 120$ ft
- Rooftop Solar Panels for Buildings of All Heights with Flat Roofs or Gable or Hip Roof with Slopes Less than 7°

ASCE 7-16

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Topics:



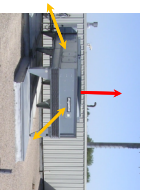
- WIND LOADS ON:
- Rooftop Equipment
 - Solar PV
 - Rooftop Screenwalls
 - Freestanding Walls and Signs
 - Tall Parapets
 - Canopies

13

Rooftop Equipment

MWFRS: 29.4.2, 29.4.3

- $F_h = q_h(GC_p)A_f, 1.0 \leq (GC_p) \leq 1.9$
- $F_v = q_h(GC_p)A_f, 1.0 \leq (GC_p) \leq 1.5$

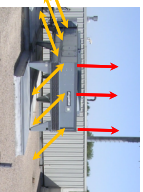


14

Rooftop Equipment

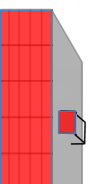
C&C: 30.10

- $P_h = q_h(GC_p), 1.0 \leq (GC_p) \leq 1.9$
- $P_v = q_h(GC_p), 1.0 \leq (GC_p) \leq 1.5$

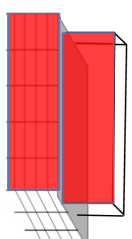


15

GC_p



$GC_p=1.9$
Area < 10% of
windward wall

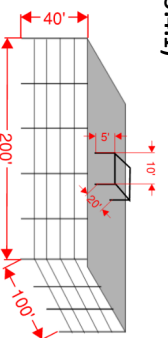


$GC_p \rightarrow 1.0$
Area = windward
wall

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Rooftop Equipment for Buildings
(MWFRS) (Section 29.4.1)

- Office Building
 - Wichita, KS (V=115mph)
 - L = 200 ft., B = 100 ft.
 - Roof Height: h = 40 ft.
 - Roof Slope, Flat: 0.25:12
 - Exposure Category: C
- Mechanical Unit Plan Dimensions: 10' wide x 20' long
 RTU Height: 4' over 1' tall curb
 Projected Height: 4+1=5'



Lateral Force: $F_h = q_h(GC_p)A_f$ (Eq 29.5-2)
 Vertical Force: $F_v = q_h(GC_p)A_f$ (Eq 29.5-3)

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Calculate q_h
 q_h calculated at mean roof height of building
 $q_h = 0.00256K_z K_{zt} K_d V^2$

Find K_{zt}
 K_z @ h = 40', Exposure C $K_z = 1.04$ (Table 29.3-1)

Height above ground level, z	Exposure			
	B	C	D	
h	(m)			
0-15	(0-4.0)	0.57	0.85	1.05
20	(6.1)	0.62	0.90	1.08
25	(7.6)	0.66	0.94	1.12
30	(9.1)	0.70	0.98	1.15
40	(12.2)	0.76	1.04	1.25

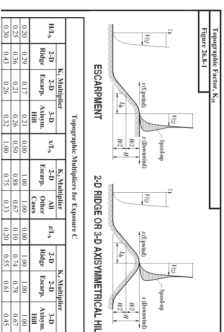
18

Find K_d for building $K_d = 0.85$ (Table 26.6-1)

Structure Type	Interim Factor K_d
Buildings With Wind Force Resisting System As Specified in Building Code	0.85
Architectural Structures	0.85
Chimneys, Tanks, and Similar Structures	0.90
Manufacturing Buildings	0.85
Solid Precast/Concrete Walls and Solid Precast/Concrete and Attached Frames	0.85
Open Signs and Similar Framework	0.85
Truss Structures, rectangular Member cross sections	0.95

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Find K_{zt}
Assume $K_{zt} = 1.0$

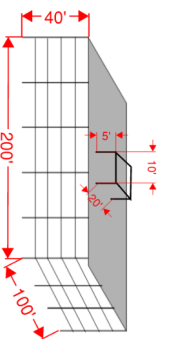


20

Find q_h

$$q_h = 0.00256K_z K_{zt} K_d V^2$$

$$q_h = 0.00256(1.04)(1.00)(0.85)(115)^2 = 29.9 \text{ psf}$$



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HORIZONTAL WIND FORCE

Check projected area of unit compared with projected area of building:

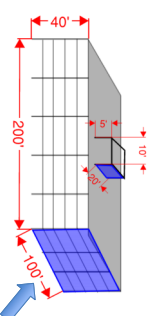
$$B^*h = 100(40) = 4,000 \text{ ft}^2$$

$$A_p = 20(5) = 100 \text{ ft}^2$$

$$A_p < 0.1Bh: 100 \text{ ft}^2 < 400 \text{ ft}^2$$

$$GC_f = 1.9$$

$(GC_f) = 1.9$ for rooftop structures and equipment with A_p less than $(0.1Bh)$. (GC_f) shall be permitted to be reduced linearly from 1.9 to 1.0 as the value of A_p is increased from $(0.1Bh)$ to (Bh) .



22

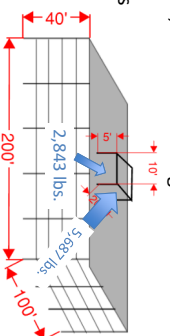
HORIZONTAL WIND FORCE

$$F_h = (29.9 \text{ psf})(1.9)(A_p) = 56.9 \text{ psf } (A_p) \text{ (Equation 29.5-2)}$$

$$F_h = (56.9 \text{ psf})(100 \text{ ft}^2) = 5,687 \text{ Lbs. } // \text{ to long side}$$

$$F_h = (56.9 \text{ psf})(50 \text{ ft}^2) = 2,843 \text{ Lbs. } \perp \text{ to long side}$$

Horizontal wind forces applied to geometric center of vertical projected plane of unit.



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VERTICAL WIND FORCE

Check projected area of roof compared with that of building:

$$B^*L = 100(200) = 20,000 \text{ ft}^2$$

$$A_p = 20(10) = 200 \text{ ft}^2$$

$$A_p < 0.1Bh: 200 \text{ ft}^2 < 2,000 \text{ ft}^2$$

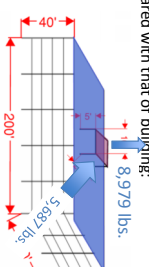
$$GC_f = 1.5$$

$$F_p = (29.9 \text{ psf})(1.5)(200 \text{ ft}^2) = 8,979 \text{ Lbs. (Equation 29.5-2)}$$

Vertical wind forces applied at geometric center of horizontal plane

$(GC_f) = 1.5$ for rooftop structures and equipment with A_p less than $(0.1Bh)$. (GC_f) shall be permitted to be reduced linearly from 1.5 to 1.0 as the value of A_p is increased from $(0.1Bh)$ to (Bh) .

Note: The UPLIFT pressure acts SIMULTANEOUSLY with either the parallel or perpendicular lateral pressure.



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**Rooftop Equipment for Buildings (C&C)
(Section 30.11)**

Loads for Designing the Equipment cabinet enclosure

Lateral C & C pressures:

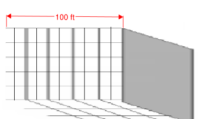
- $F_w = 5,687$ Lbs. (from previous)
- $F_v = 8,979$ Lbs. (from previous)

C & C Lateral Loads: $F_w/A = 5,687$ Lbs./100 ft² = 56.9 psf
 → Load is applied toward or away from unit on all sides.

C & C Vertical Loads: $F_v/A = 8,979$ Lbs./200 ft² = 44.9 psf
 → Load is applied only in the Upward direction, away from the top of the unit

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**What about rooftop equipment for
h>60 feet?**



ASCE 7-16

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Topics:

WIND LOADS ON:

- Rooftop Equipment
- Solar PV
- Rooftop Screenwalls
- Freestanding Walls and Signs
- Tall Parapets
- Canopies



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Solar PV



ASCE 7-16

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Solar PV

- The SEAOC PV committee was formed in September 2011.
- Goal: To address the lack of requirements in the code for PV systems.



2012:

- PV1-2012: Seismic Design
- PV2-2012: Wind Design → ASCE 7-16 incorporates and adopts PV2-2012

2016:

- PV2-2016: Supersedes PV2-2012
- References ASCE 7-16
- Knowledge from research since 2012
- Updated terminology, effective wind area determination, wind tunnel requirements
- In some cases, "recommended additional requirements" where the ASCE 7-16 requirements may not be adequate.

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Guide DOES Cover:

- Arrays with tilted panels on flat or low-slope roof buildings (Section 4)
- Parallel-to-roof (flush-mounted) arrays on sloped roofs (Section 5)
- Ground-mounted solar arrays (Section 8)

Guide DOES NOT Cover:

- Roof-mounted systems with tilted panels that are not low-profile
- Arrays on other roof shapes (e.g. hip, gable, saw-tooth, etc.)



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- Phase 1** IBHS Research Center & ASHRAE
Testing: Preliminary Findings
- Equipment height above top of screen increases wind loads.
 - Fully enclosed configurations lower wind loads.
 - Partially enclosed screen configurations do not provide significant wind load reduction.
 - Screen type does not significantly change wind loads.

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- Phase 2** IBHS Research Center & ASHRAE
Testing
- Evaluate wind loads on screenwalls themselves.

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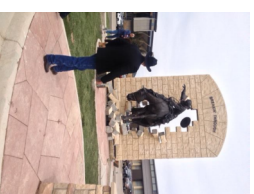
Topics:

WIND LOADS ON:

- Rooftop Equipment
- Solar PV
- Rooftop Screenwalls
- Freestanding Walls and Signs
- Tall Parapets
- Canopies



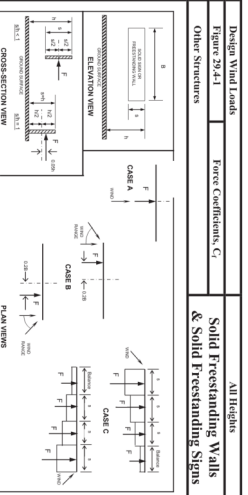
**Solid Freestanding
Walls and Signs**



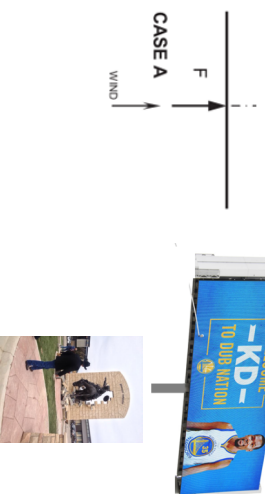
39

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Solid Freestanding Walls and Signs



Solid Freestanding Walls and Signs



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Solid Freestanding Walls and Signs

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Solid Sign at Ground Level

(Exposure C, 115 mph)
 $F = q_h G C_p A_s$ (Eq 29.4-1)

30' Wide x 10' High $\rightarrow s=10'$, $B=30'$, $h=10'$

Your Ad Here

$K_{h1} = 0.85$ (Table 29.3-1)
 $K_{h2} = 0.85$ (Table 26.6-1)
 $K_{z1} = 1.0$ (assumed)

$q_{h1} = 0.00256 K_{h1} K_{z1} K_{y1} V^2$
 $q_{h1} = 0.00256(0.85)(1.00)(0.85)(115)^2 = \mathbf{24.5 \text{ psf}}$
 $G = 0.85$ (Section 26.9 – Rigid Structure)

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Figure 29.4-1 for C_p

$F = q_h G C_p A_s$ (Eq 29.4-1)

$F = (24.5 \text{ psf})(0.85)(1.375)A_s = 28.59 \text{ psf} A_s$

$A_s = B \times s = 30' \times 10' = 300 \text{ ft}^2$

$F = 28.6 \text{ psf} \times A_s = 28.6 \text{ psf}(300 \text{ ft}^2) = 8,577 \text{ Lbs.}$

For CASE A, Load is applied at plan center and at $(s/2) + (0.05h) = 5.5'$ above base.

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Solid Freestanding Walls and Signs

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Figure 29.4-1 for C_p

$B = \frac{30'}{10'} = 3.0$ Aspect Ratio
 $s = 10'$ Clearance Ratio
 $h = 10'$ Clearance Ratio

Clearance Ratio, s/h	C_p CASE A & CASE B										
	Wind from Side 1		Wind from Side 2		Wind from Side 3		Wind from Side 4		Wind from Side 5		
<0.05	0.1	0.2	0.5	1	1.45	1.40	1.35	1.35	1.35	1.30	1.30
0.1	1.80	1.70	1.65	1.60	1.55	1.50	1.45	1.45	1.40	1.35	1.30
0.2	1.85	1.75	1.70	1.65	1.60	1.55	1.50	1.45	1.45	1.40	1.40
0.5	1.90	1.85	1.75	1.70	1.65	1.60	1.55	1.55	1.55	1.55	1.55
0.7	1.95	1.85	1.75	1.70	1.65	1.60	1.55	1.55	1.55	1.55	1.55
1.0	1.95	1.85	1.80	1.75	1.70	1.65	1.60	1.60	1.60	1.60	1.60
0.5	1.95	1.85	1.80	1.75	1.70	1.65	1.60	1.60	1.60	1.60	1.60
0.2	1.95	1.80	1.85	1.80	1.80	1.80	1.80	1.80	1.80	1.80	1.80
<0.05	1.95	1.90	1.85	1.85	1.80	1.80	1.80	1.80	1.80	1.80	1.80

For B/s = 2, $C_p = 1.40$ → Interpolating for B/s 3.0, $C_p = 1.375$
 For B/s = 4, $C_p = 1.35$

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For CASE B, load is applied at 5.5' above base and at 0.2B offset from either side of plan centerline.
 $0.2B = 0.2(30') = 6.0'$ either side of plan centerline.

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Check if CASE C must be considered (note 3, Figure 29.4-1)

→ If $B/s \geq 2.0$, CASE C must be considered.

$$\frac{B}{s} = \frac{30'}{10'} = 3.0 > 2.0$$

→ Consider CASE C.

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Enter Figure 29.4-1 for C_p under CASE C $\frac{B}{s} = \frac{30'}{10'} = 3.0$

Region (horizontal distance from windward edge)	Aspect Ratio, B/s							
	2	3	4	5	6	7	8	
0 to 8	2.25	2.60	2.90	3.10*	3.30*	3.40*	3.55*	
8 to 28	1.50	1.70	1.90	2.00	2.15	2.25	2.30	
28 to 38		1.15	1.30	1.45	1.55	1.65	1.70	
38 to 108			1.10	1.05	1.05	1.05	1.05	

Footnote 4:

0-8 → $C_p = 2.60$ from 0 to 10 ft.
 8-28 → $C_p = 1.70$ from 10 to 20 ft.
 28-38 → $C_p = 1.15$ from 10 to 30 ft.

For CASE C, where $s/h > 0.8$, C_p may be multiplied by reduction factor $(1.8 - s/h)$.
 $s/h = 1.0 > 0.8$
 → $(1.8 - s/h) = (1.8 - 1.0) = 0.8$

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$F = q_h G C_p A_s$ (Eq 29.4-1)

$F_1 = (24.5 \text{ psf})(0.85)(2.60)(0.8)(10 \times 10) = 4,324 \text{ lbs.}$
 $F_2 = (24.5 \text{ psf})(0.85)(1.70)(0.8)(10 \times 10) = 2,828 \text{ lbs.}$
 $F_3 = (24.5 \text{ psf})(0.85)(1.15)(0.8)(10 \times 10) = 1,913 \text{ lbs.}$

For CASE C, apply F_1 , F_2 , and F_3 at plan centerline of each length, s .
 Apply F_1 , F_2 and F_3 at 5.5' above base.

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Solid Freestanding Walls and Signs: ASCE 7-16

- New research and provision cover deeper signs (electronic).
- Minor revisions to the provisions.

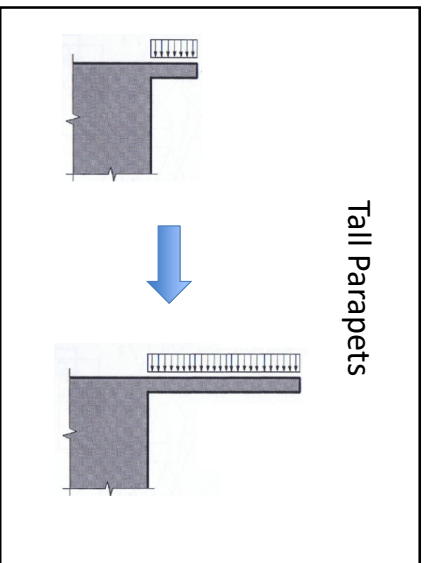
53

Topics:

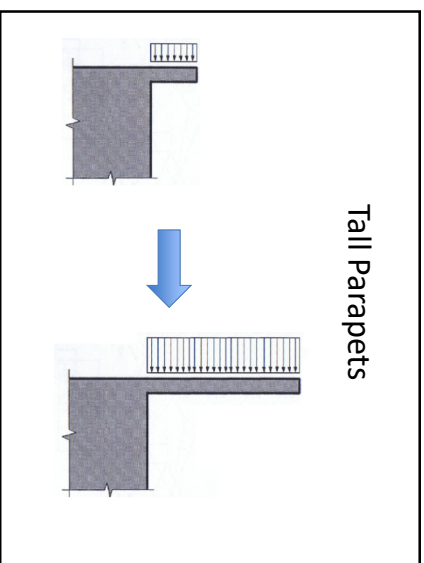
WIND LOADS ON:

- Rooftop Equipment
- Solar PV
- Rooftop Screenwalls
- Freestanding Walls and Signs
- Tall Parapets
- Canopies

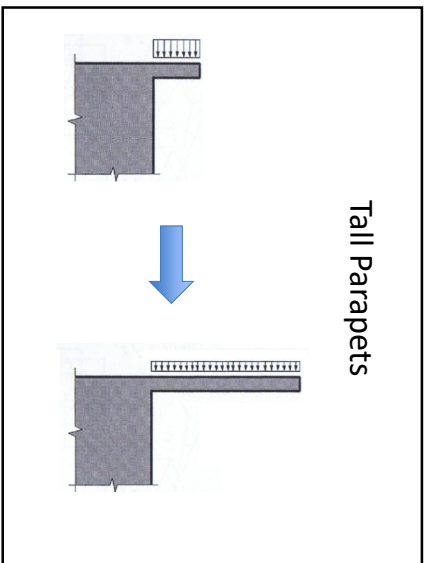
54



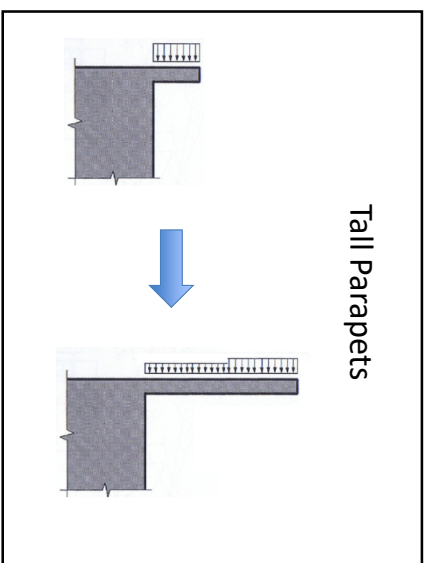
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History of Parapet Design

- Before ASCE 7-02 there were no provisions for wind loads on parapets.
- ASCE 7-02 a method was introduced based on *“the committee’s collective experience, intuition, and judgement.”*
- ASCE 7-05 provisions were updated with research from University of Western Ontario and Concordia University.

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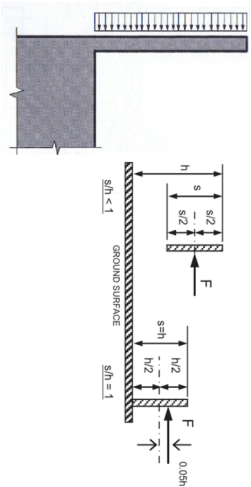
Parapet Research To-Date

- There are many studies on parapets effects on the roof loads.
- Due to instrumentation limitations, there are limited studies on wind forces on the parapet itself.
- Results of tests suggest wind loads on parapets are independent of parapet height¹

Reference: Wind Loads on Parapets: Part 2, Structural and Local Cladding Loading on the Parapet Itself, C. Wang, G. Koppell, D. Saffry, BIVT-SSJ-2002/June 2001

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Tall Parapets



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Topics:

WIND LOADS ON:

- Rooftop Equipment
- Solar PV
- Rooftop Screenwalls
- Freestanding Walls and Signs
- Tall Parapets
- Canopies



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Canopies and Awnings

External Pressure Coefficient, C_p

- One of the oldest figures of ASCE 7.
- Helps us understand wind behavior.

Surface	Wall Bracing Coefficient, C_{br}	Top Wind
Windward Wall	All Values	0.8
	0.1	-0.5
Leeward Wall	2	-0.3
	24	-0.2
Side Wall	All Values	-0.7
		0

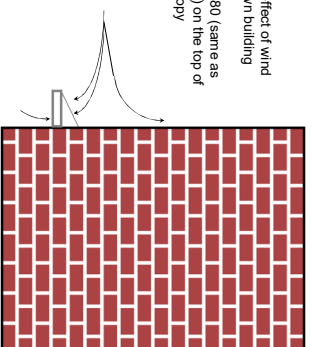
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Canopies and Awnings

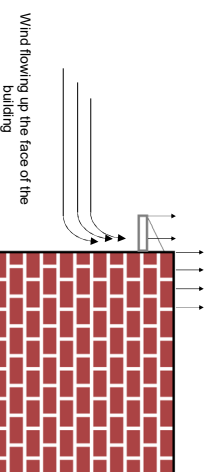
Down-draft effect of wind flowing down building

→ Use $C_p = -0.80$ (same as windward wall) on the top of canopy



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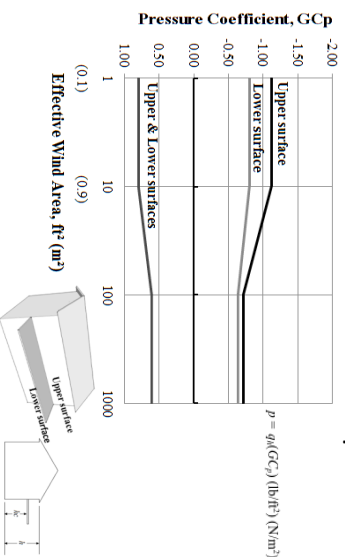
Canopies and Awnings



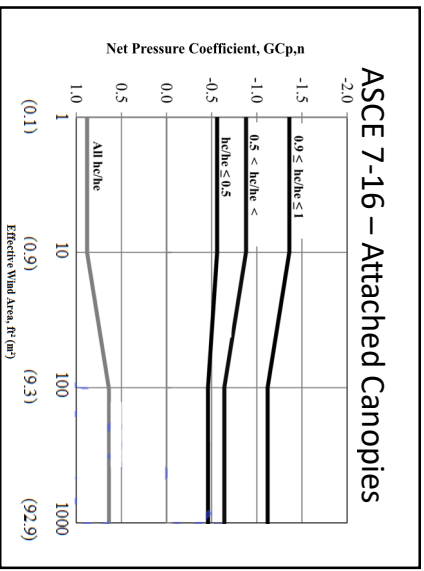
→ Use $C_p = -0.80$ (same as windward wall) on the bottom of the canopy plus the roof uplift on the top.

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ASCE 7-16 – Attached Canopies



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Additional Resources

- Wind Loads for Petrochemical and Other Industrial Facilities
- SEAOC Wind Design for Low-Profile Solar Photovoltaic Arrays on Flat Roofs

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Additional Resources

- Guide Specifications for Design of Metal Flagpoles, ANSI/NAAMM FP1001-97, 4th Edition
- Structural Standards for Steel Antenna Towers and Antenna Supporting Structures, ANSI/TIA-222-G

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Additional Resources

- Wind Loads on Small Roof-Mounted Air-Conditioning Units, IBHS Research Center

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Summary

- There are attempts to address commonly encountered conditions (canopies, $h > 60'$, screenwalls, solar-PV).
- Even when the code doesn't address an issue, we can extrapolate to find a solution.
- Use resources from related industries for guidance on non-building wind loads.

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 eguglielmo@martinmartin.com
 415-814-0030

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